

December 15, 2011

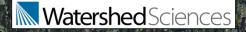
Submitted to:

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LIDAR REMOTE SENSING DATA COLLECTION:

DOGAMI, FS NEWBERRY STUDY AREA

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1. Overview

1.1 Study Area

Watershed Sciences, Inc. has collected Light Detection and Ranging (LiDAR) data of the FS Newberry 2 Study Area for the Oregon Department of Geology and Mineral Industries (DOGAMI). The area of interest (AOI) totals 444 square miles (284,030 acres) and the total area flown (TAF) covers 462 square miles (296,074 acres). The TAF acreage is greater than the original AOI acreage due to buffering and flight planning optimization (Figure 1.1 below). This report reflects all data and cumulative statistics for the overall LiDAR survey. DOGAMI data are delivered in OGIC (HARN): Projection: Oregon Statewide Lambert Conformal Conic; horizontal and vertical datum: NAD83 (HARN)/NAVD88 (Geoid03); units: International Feet.

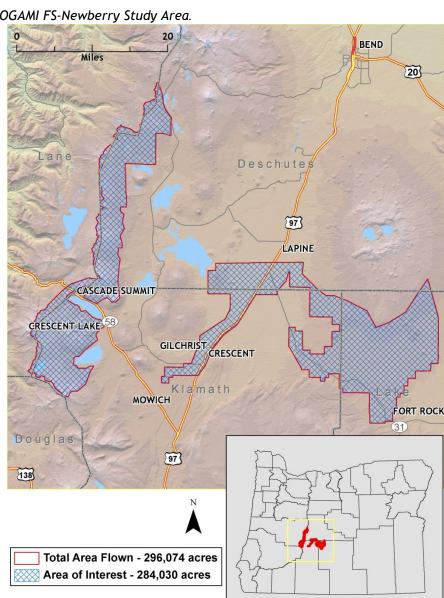


Figure 1.1. DOGAMI FS-Newberry Study Area.

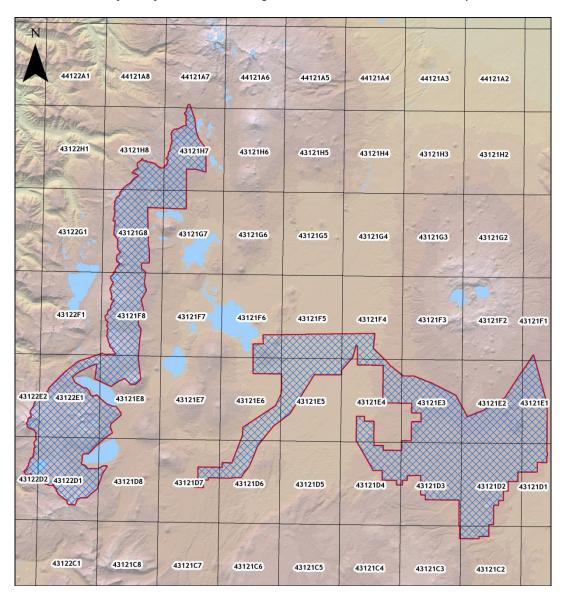
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1.2 Area Delivered to Date

Total delivered acreage detailed below.

DOGAMI FS Newberry 2 Study Area				
	Delivery Date Acquisition Dates AOI Acres TAF Acres			
Delivery Area	December 15, 2011	July 31, 2011 - August 10, 2011	284,030	296,074

Figure 1.3. FS Newberry Study Area, illustrating the delivered 7.5 minute USGS quads.



2. Acquisition

2.1 Airborne Survey Overview - Instrumentation and Methods

The LiDAR survey utilized a Leica ALS60 and an ALS50 Phase II sensor mounted in Cessna Caravan 208B. The Leica systems were set to acquire $\geq 75,000$ and $\geq 96,000$ laser pulses per second (i.e. 75 kHz pulse rate) and flown at 900 and 1300 meters above ground level (AGL), capturing a scan angle of $\pm 14-15^{\circ}$ from nadir¹. These settings are developed to yield points with an average native density of ≥ 8 points per square meter over terrestrial surfaces. The native pulse density is the number of pulses emitted by the LiDAR system. Some types of surfaces (i.e. dense vegetation or water) may return fewer pulses than the laser originally emitted. Therefore, the delivered density can be less than the native density and lightly variable according to distributions of terrain, land cover and water bodies.





The Cessna Caravan is a powerful, stable platform, which is ideal for the often remote and mountainous terrain found in the Pacific Northwest. The Leica ALS60 sensor head installed in the Caravan is shown on the right.

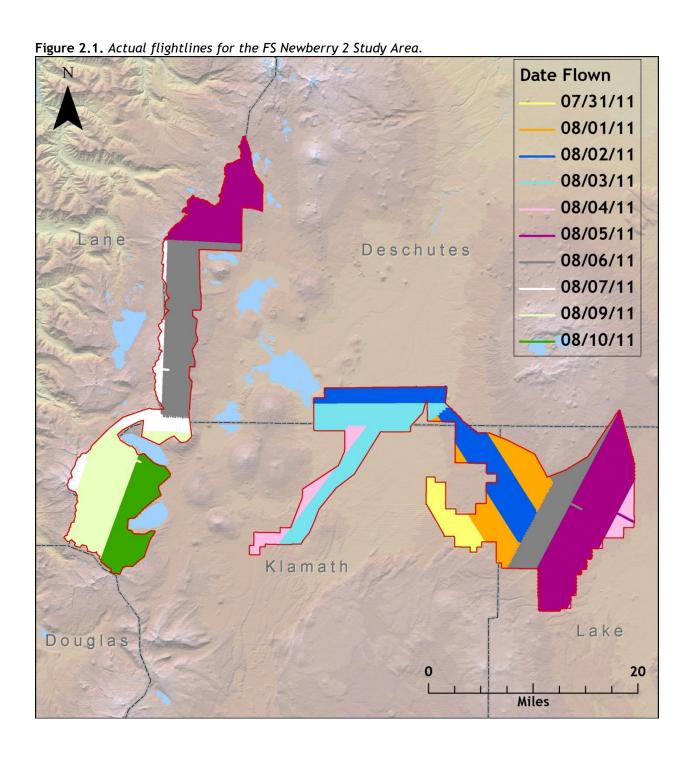
Table 2.1 LiDAR Survey Specifications

Sensors	Leica ALS60 and ALS50 Phase II
Survey Altitude (AGL)	900 m and 1300 m
Pulse Rate	>75 kHz and >96KHz
Pulse Mode	Single
Mirror Scan Rate	54 Hz and 61.1 Hz
Field of View	28° (±14° from nadir) dual laser system,
	30° (±14° from nadir) single laser system
Roll Compensated	Up to 15°
Overlap	100% (50% Side-lap)

The study area was surveyed with opposing flight line side-lap of $\geq 50\%$ ($\geq 100\%$ overlap) to reduce laser shadowing and increase surface laser painting. The system allows up to four range measurements per pulse, and all discernible laser returns were processed for the output dataset.

To solve for laser point position, it is vital to have an accurate description of aircraft position and attitude. Aircraft position is described as x, y and z and measured twice per second (2 Hz) by an onboard differential GPS unit. Aircraft attitude is measured 200 times per second (200 Hz) as pitch, roll and yaw (heading) from an onboard inertial measurement unit (IMU). **Figure 2.1** shows the flight lines completed for current processing.

¹ Nadir refers to the perpendicular vector to the ground directly below the aircraft. Nadir is commonly used to measure the angle from the vector and is referred to a "degrees from nadir".



2.2 Ground Survey - Instrumentation and Methods

During the LiDAR survey, static (1 Hz recording frequency) ground surveys were conducted over either known or set monuments. Monument coordinates are provided in Table 2.2 and shown in Figure 2.2 for the AOI. After the airborne survey, the static GPS data are processed using triangulation with Continuously Operatating Reference Stations (CORS) and checked using the Online Positioning User Service (OPUS²) to quantify daily variance. Multiple sessions are processed over the same monument to confirm antenna height measurements and reported position accuracy. Control monuments are located within 13 nautical miles of the survey area(s). Indexed by time, these GPS data records are used to correct



the continuous onboard measurements of aircraft position recorded throughout the mission.



2.2.1 Instrumentation

For this study area all Global Navigation Satellite System (GNSS³) survey work utilizes a Trimble GPS receiver model R7 with a Zephyr Geodetic antenna with ground plane for static control points. The Trimble GPS R8 unit is used primarily for Real Time Kinematic (RTK) work but can also be used as a static receiver. For RTK data, the collector begins recording after remaining stationary for 5 seconds then calculating the pseudo range position from at least three epochs with the relative error under 1.5 cm horizontal and 2 cm vertical. All GPS measurements are made with dual frequency L1-L2 receivers with carrier-phase correction.

2.2.2 Monumentation

Whenever possible, existing and established survey benchmarks shall serve as control points during LiDAR acquisition including those previously set by Watershed Sciences. In addition to NGS, the county surveyor's offices and the Oregon Department of Transportation (ODOT) often establish their own benchmarks. NGS benchmarks are preferred for control points. In the absence of NGS benchmarks, county surveys, or ODOT monumentation, Watershed Sciences produces our own monuments. These

monuments are spaced at a minimum of one mile and every effort is made to keep these monuments within the public right of way or on public lands. If monuments are required on private property, consent from the owner is required. All monumentation is done with 5/8" x 30" rebar topped with a 2" diameter aluminum cap stamped "Watershed Sciences, Inc." plus the year and monument id.

² Online Positioning User Service (OPUS) is run by the National Geodetic Survey to process corrected monument positions.

³ GNSS: Global Navigation Satellite System consisting of the U.S. GPS constellation and Soviet GLONASS constellation

2.2.3 Methodology

Each aircraft is assigned a ground crew member with two R7 receivers and an R8 receiver. The ground crew vehicles are equipped with standard field survey supplies and equipment including safety materials. All control points are observed for a minimum of two survey sessions lasting no fewer than 2 hours. At the beginning of every session the tripod and antenna are reset, resulting in two independent instrument heights and data files. Data are collected at a rate of 1Hz using a 10 degree mask on the antenna.

The ground crew uploads the GPS data to the Dropbox website on a daily basis to be returned to the office for Professional Land Surveyor (PLS) oversight, Quality Assurance/Quality Control (QA/QC) review and processing. OPUS processing triangulates the monument position using 3 CORS stations resulting in a fully adjusted position. CORPSCON⁴ 6.0.1 software is used to convert the geodetic positions from the OPUS reports. After multiple days of data have been collected at each monument, accuracy and error ellipses are calculated. This

information leads to a rating of the

monument based on FGDC-STD-007.2-1998⁵

Part 2 table 2.1 at the 95% confidence level.

All RTK measurements were made during periods with a Position Dilution of Precision (PDOP) of ≤ 3.0 and in view of at least six satellites by the stationary reference and roving receiver. RTK positions are collected on 20% of the flight lines and on bare earth locations such as paved, gravel or stable dirt roads, and other locations where the ground is clearly visible (and is likely to remain visible) from the sky during the data acquisition and RTK measurement period(s). In order to facilitate comparisons with LiDAR measurements, RTK measurements are not taken on highly reflective surfaces such as center line

stripes or lane markings on roads. RTK points were taken no closer than one meter to any nearby terrain breaks such as road edges or drop offs. In addition, it is desirable to include locations that can be readily identified and occupied during subsequent field visits in support of other quality control procedures described later. Examples of identifiable locations would include manhole and other flat utility structures that have clearly indicated center points or other measurement locations. In the absence of utility structures, a PK nail can be driven into asphalt or concrete and marked with paint.

Multiple differential GPS units were used in the ground based real-time kinematic (RTK)



portion of the survey. To collect accurate ground surveyed points, a GPS base station unit was set up over monuments to broadcast a kinematic correction to a roving GPS unit. The ground crew used a

 $^{^4}$ U.S. Army Corps of Engineers , Engineer Research and Development Center Topographic Engineering Center software

⁵ Federal Geographic Data Committee Draft Geospatial Positioning Accuracy Standards

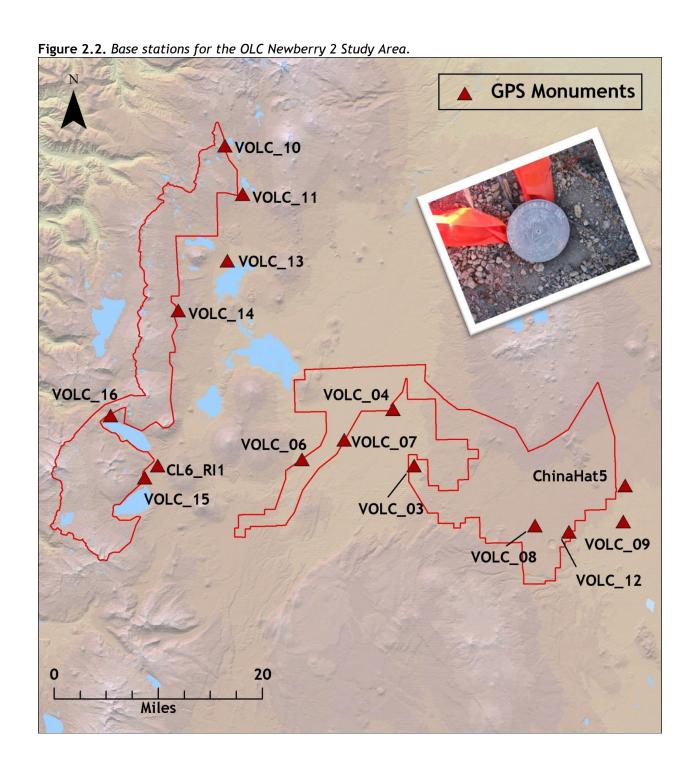
roving unit to receive radio-relayed kinematic corrected positions from the base unit. This RTK survey allowed precise location measurement ($\sigma \le 1.5$ cm). Figure 2.3 shows subsets of these RTK locations.

Table 2.2. Base Station Surveyed Coordinates, (NAD83/NAVD88, OPUS corrected) used for kinematic post-processing of the aircraft GPS data for the FS Newberry 2 Study Area.

	Datum NAD83 (HARN)		GRS80
Base Stations ID	Latitude (North)	Longitude (West)	Ellipsoid Height (m)
VOLC_03	43 31 24.0462	121 27 23.724	1304.933
VOLC_04	43 36 07.8928	121 29 41.922	1278.179
VOLC_06	43 32 05.588	121 40 17.044	1307.509
VOLC_07	43 33 39.4821	121 35 23.777	1293.420
VOLC_08	43 25 36.8422	121 09 48.985	1380.625
VOLC_09	43 26 22.654	121 03 34.901	1339.671
VOLC_10	43 58 22.564	121 48 35.502	1480.588
VOLC_11	43 54 18.048	121 46 38.177	1436.832
VOLC_12	43 26 12.312	121 13 41.185	1401.4965
VOLC_13	43 48 45.748	121 48 30.191	1342.1325
VOLC_14	43 44 40.981	121 54 13.221	1578.753
VOLC_15	43 3044.535	121 58 21.533	1434.338
VOLC_16	43 35 58.584	122 02 11.662	1541.63
ChinaHat5	43 29 23.4228	121 03 15.988	1354.254
CL6_RI1	43 31 45.454	121 56 50.296	1541.63



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For data delivered to date, 2,951 RTK (Real-time kinematic) points were collected in the study area. Figures 2.3 shows detailed views of selected RTK locations for all areas.

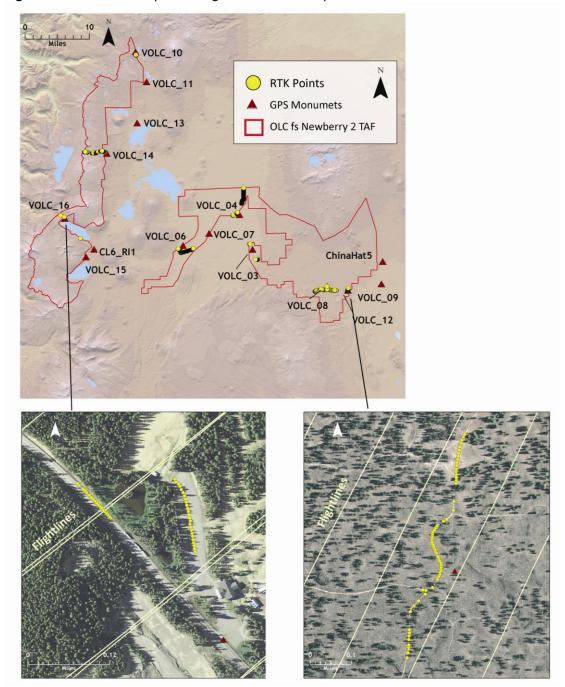


Figure 2.3 Selected RTK point images are NAIP orthophotos.

3. Accuracy

3.1 Relative Accuracy

Relative Accuracy Calibration Results

Relative accuracy refers to the internal consistency of the data set and is measured as the divergence between points from different flightlines within an overlapping area. Divergence is most apparent when flightlines are opposing. When the LiDAR system is well calibrated the line to line divergence is low (<10 cm). Internal consistency is affected by system attitude offsets (pitch, roll and heading), mirror flex (scale), and GPS/IMU drift.

Relative accuracy statistics are based on the comparison of 701 flightlines and over 10 billion points. Relative accuracy is reported for the portion of the study area shown in **Figure 3.1** below.

- o Project Average = 0.12ft (0.04m)
- Median Relative Accuracy = 0.11 ft (0.03 m)
- o 1σ Relative Accuracy = 0.14 ft (0.04m)
- \circ 2 σ Relative Accuracy = 0.19 ft (0.06 m)

Figure 3.1. Relative Accuracy Covered Area.

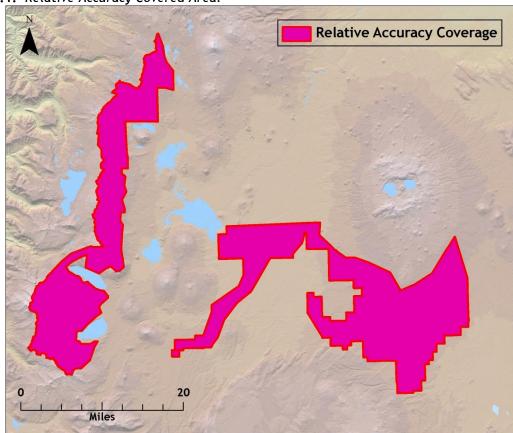


Figure 3.2. Statistical relative accuracies, non slope-adjusted.

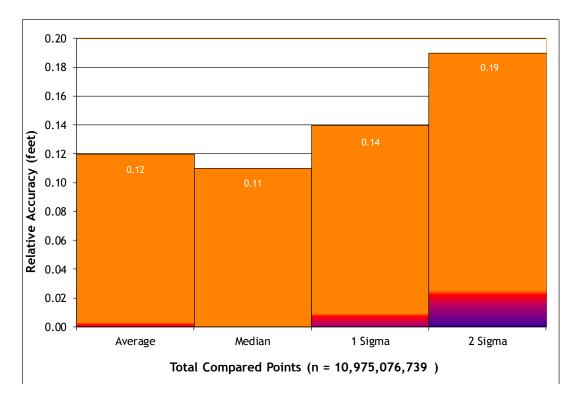
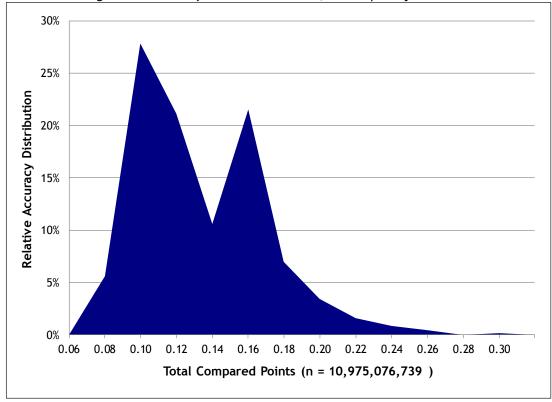


Figure 3.3. Percentage distribution of relative accuracies, non slope-adjusted.



3.2 Absolute Accuracy

Absolute accuracy compares known RTK ground survey points to the closest laser point. For the Newberry Study Area, 2,951 RTK points were collected for data in the study area. Absolute accuracy is reported for the portion of the study area shown in Figure 3.4 and reported in Table 3.1 below. Histogram and absolute deviation statistics are reported in Figures 3.5 and 3.6.

Table 3.1. Absolute Accuracy - Deviation between laser points and RTK survey points.

Sample Size (n): 2,951		
Root Mean Square Error (RMSE): 0.16 ft (0.05m)		
Standard Deviations Deviations		
1 sigma (σ): 0.16ft (0.05 m)	Minimum Δz: -0.67 ft (-0.20m)	
2 sigma (σ): 0.32 ft (0.10 m)	Maximum Δz: 0.62 ft (0.19 m)	
	Average Δz: - 0.04 ft (-0.01 m)	



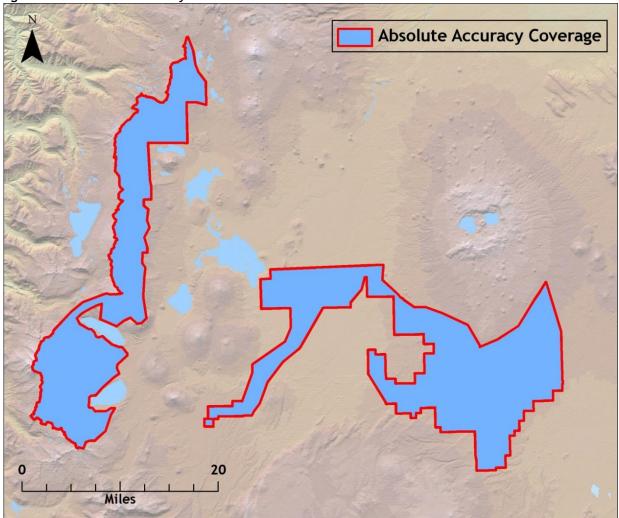


Figure 3.5. FS Newberry Study Area histogram statistics

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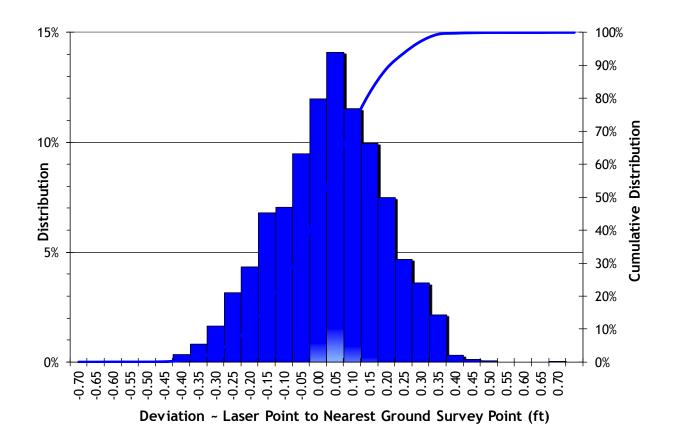
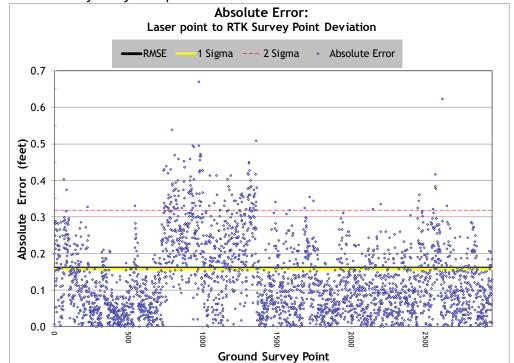


Figure 3.6. FS Newberry Study Area point absolute deviation statistics.



4. Data Density/Resolution

4.1 Density Statistics

Some types of surfaces (i.e. dense vegetation or water) may return fewer pulses than the laser originally emitted. Therefore, the delivered density can be less than the native density and vary according to terrain, land cover and water bodies. Density histograms and maps (Figures 4.1 - 4.4) have been calculated based on first return laser point density and ground-classified laser point density.

Table 4.1. Average density statistics for the FS Newberry Study Area.

Average Pulse	Average Pulse	Average Ground	Average Ground
Density	Density	Density	Density
(per square ft)	(per square m)	(per square ft)	(per square m)
0.95	10.2	0.15	1.65

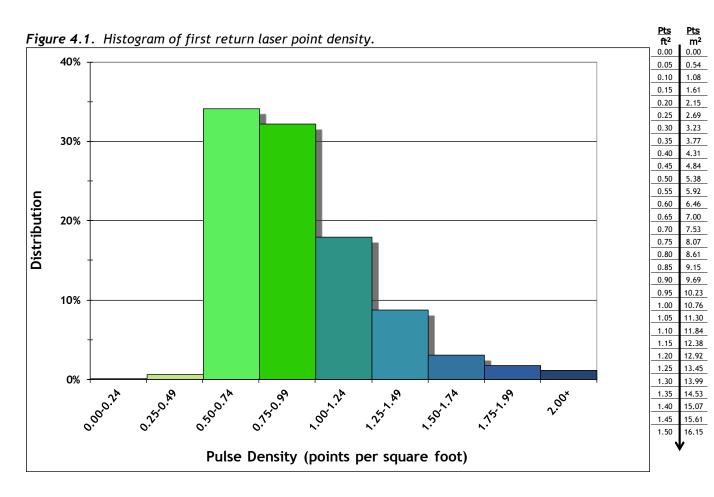
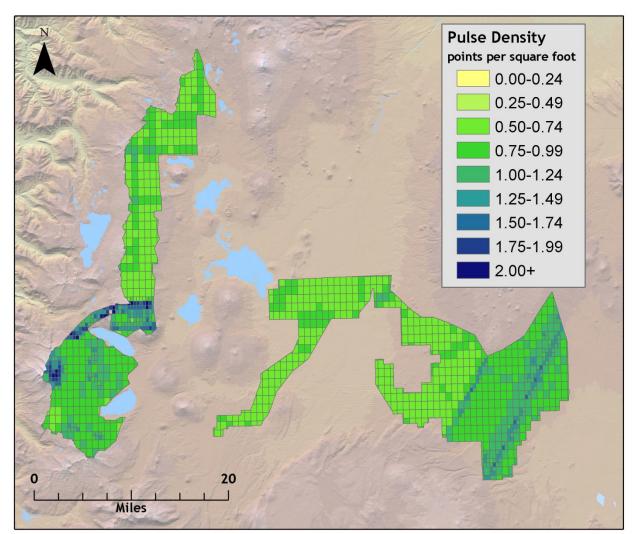


Figure 4.2. First return laser point densities per 0.75' USGS Quad.



Pts ft²	Pts m ²
0.00	0.00
0.05	0.54
0.10	1.08
0.15	1.61
0.20	2.15
0.25	2.69
0.30	3.23
0.35	3.77
0.40	4.31
0.45	4.84
0.50	5.38
0.55	5.92
0.60	6.46
0.65	7.00
0.70	7.53
0.75	8.07
0.80	8.61
0.85	9.15
0.90	9.69
0.95	10.23
1.00	10.76
1.05	11.30
1.10	11.84
1.15	12.38
1.20	12.92
1.25	13.45
1.30	13.99
1.35	14.53
1.40	15.07
1.45	15.61
1.50	16.15
1	y

Ground classifications were derived from ground surface modeling. Classifications were performed by reseeding of the ground model where it was determined that the ground model failed, usually under dense vegetation and/or at breaks in terrain, steep slopes and at bin boundaries.

Figure 4.3. Histogram of ground-classified laser point density.

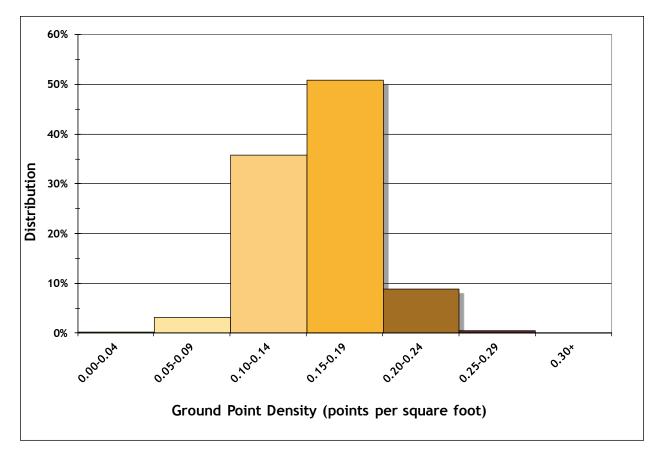
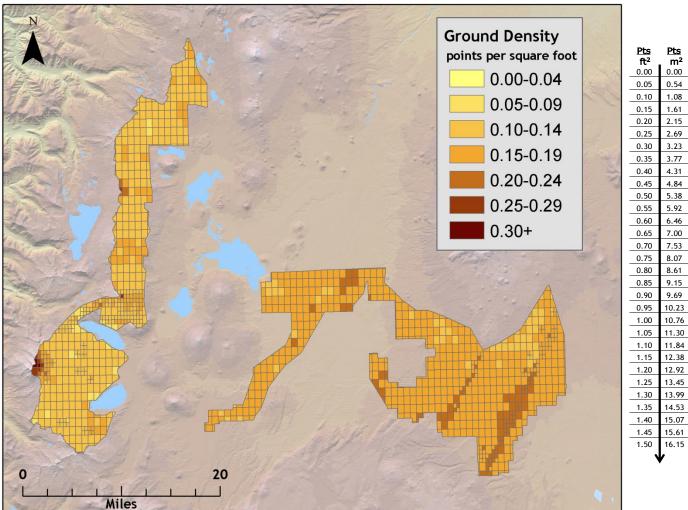


Figure 4.4. Ground-classified laser point density per 0.75' USGS Quad for data delivered to date.



5. Certifications

Watershed Sciences provided LiDAR services for the FS Newberry 2 study area as described in this report.

I, Mathew Boyd, have reviewed the attached report for completeness and hereby state that it is a complete and accurate report of this project.

Mathew Boyd Principal

Watershed Sciences, Inc.

Mout Boyd

I, Christopher W. Yotter-Brown, being first dully sworn, say that as described in the Ground Survey subsection of the Acquisition section of this report was completed by me or under my direct supervision and was completed using commonly accepted standard practices. Accuracy statistics shown in the Accuracy Section have been reviewed by me to meet National Standard for Spatial Data Accuracy.

Christopher W. Yotter-Brown, PLS Oregon & Washington Watershed Sciences, Inc Portland, OR 97204

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6. Selected Imagery

Figure 5.1. Lakeview Mountain with Diamond Peak in the distance, located three miles southeast of Odell Lake, Oregon. View to the West.

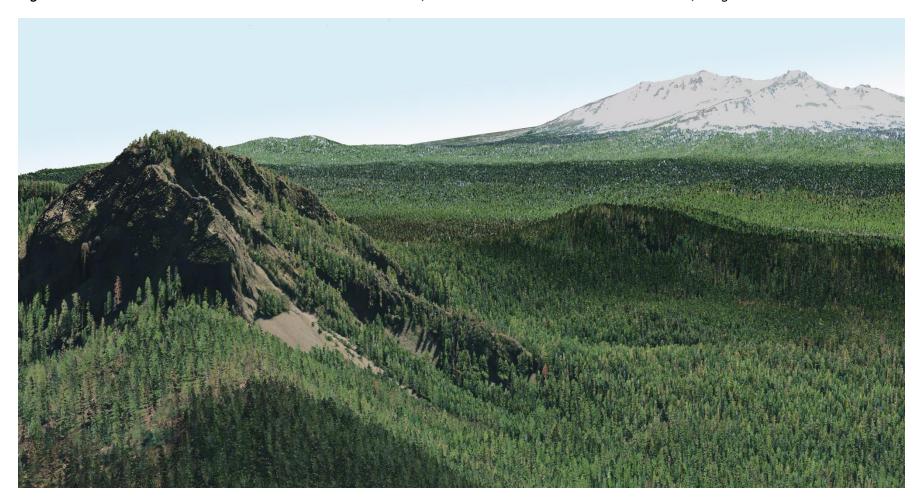
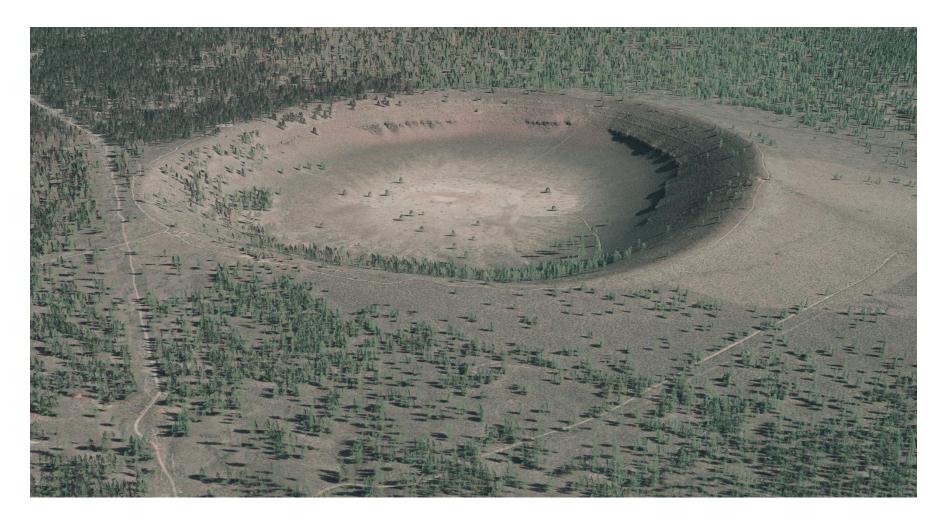


Figure 5.2. Hole in the Ground terrain depression, located 60 miles northeast of Crater Lake. View to the North.



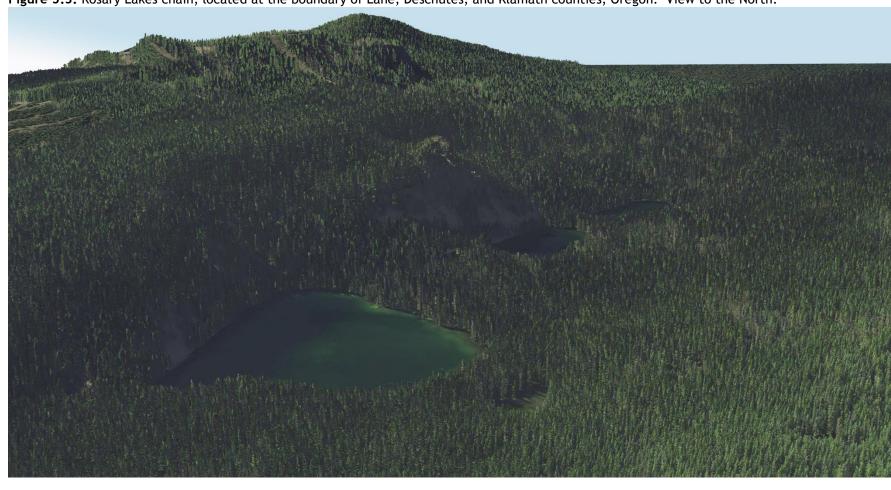


Figure 5.3. Rosary Lakes chain, located at the boundary of Lane, Deschutes, and Klamath counties, Oregon. View to the North.

Figure 5.4. (cover). Maiden Peak, located four miles north of Odell Lake, Oregon. View to the Southwest.

